

2 March 2020

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Re: Georges Cove Marina at 146 Newbridge Road, Moorebank - Response to RMS matters

Dear Felix,

This letter has been prepared in response to the traffic related matters that have been raised by Roads & Maritime Services (RMS) (now Transport for NSW (TfNSW)) in their letter dated 8 November 2019 in regard to construction and operation of Georges Cove Marina located at 146 Newbridge Road, Moorebank (TNSW Ref: SYD18/01728/03, Council Ref: DA-611/2018).

RMS comments and our responses are provided below.

1.1 Matter 1

RMS Comment:

The submitted traffic model needs to be updated to include 2019 traffic count data in order to consider cumulative traffic growth since the lodgement of the original traffic applications for the various land uses at 146 Newbridge Road, Moorebank.

EMM Response:

A new tube count was undertaken at Brickmakers Drive near the proposed Link Road between 30 January 2020 and 5 February 2020. In addition, the Newbridge Road/Brickmakers Drive/Governor Macquarie Drive intersection was surveyed on 30 January 2020 (refer to Appendix A for traffic count data). The SIDRA model has been updated to reflect these latest traffic counts.

1.2 Matter 2

RMS Comment:

Further clarification is requested regarding the future development traffic volumes used in the submitted traffic assessment. For instance, when will these various developments be completed and using the new road connection to Brickmakers Creek [sic, presumably 'Drive']?

EMM Response:

1.2.1 Cumulative traffic generating developments

In accordance to the respective traffic studies, the generated traffic of the following proposed developments will use the Link Road off Brickmakers Drive. The following reports cover the future development traffic to/from the site:

- Moorebank Cove Residential Estate (EMM Consulting 2016);
- Moorebank Recycling Facility (Lyle Marshall & Associates 2012);
- Georges Cove Marina (EMM Consulting 2018);
- Georges Cove Marina residential development (EMM Consulting 2018); and
- B6 Corridor mixed-use development (Ason Group 2017).

Development applications for Moorebank Cove Residential Estate and the B6 Corridor mixed-use development are currently underway. A planning proposal application has been submitted to allow future residential development associated with the marina. The Moorebank Recycling Facility has been approved. In our understanding, that sufficient work has been completed on the facility, that it is considered to have 'physically commenced'. However, construction work has ceased and it is not known when it will recommence. Thus, cumulative traffic considers the scenario where all these developments have been constructed and in operation.

1.2.2 Traffic generation rates used in previous EMM reports

The Moorebank Cove Residential Estate will be a low-density residential development. A traffic generation rate of 0.85 per dwelling in the peak hours has been adopted, as per the RTA (now TfNSW) *Guide to Traffic Generating Developments*.

In accordance with *Guide to Traffic Generating Developments*, the following traffic generation rates have been adopted for the Georges Cove Marina development:

- 0.485 per medium-density residential dwelling;
- 0.14 per dry boat storage berth;
- 0.14 per wet berth marina; and
- 2 per 100 m² commercial GFA (only for the afternoon peak hour).

1.2.3 Summary of traffic generations and distributions

The traffic generation and distributions of these developments were discussed in their respective traffic reports, a summary of these traffic generations is provided in Table 1.

Table 1 Development traffic generation using the new Link Road

	Peak hour	Description	Traffic generation	Inbound	Outbound
Moorebank Cove	AM	470 day.!!!	152	30	122
	PM	179 dwellings	152	91	61
Moorebank Recycling Facility	AM		43	23	20

Table 1 Development traffic generation using the new Link Road

	Peak hour	Description	Traffic generation	Inbound	Outbound
	PM	Trucks delivering/ dispatching waste and dispatching products	29	13	16
Georges Cove Marina	AM	1,243 m ² commercial gross floor area (GFA), 250	61	49	12
Commercial	PM	dry storage berths, 186 marina berths	86	43	43
George Cove Marina	AM	074 111	181	37	145
Residential	PM	374 dwellings	181	108	73
Benedict B6 Corridor Mixed-	AM		296	145	151
use Development	PM	-	386	172	214
Total	AM		733	284	450
	PM	-	834	427	407

The distributions of these development traffic are presented in Figure 1 to Figure 5.

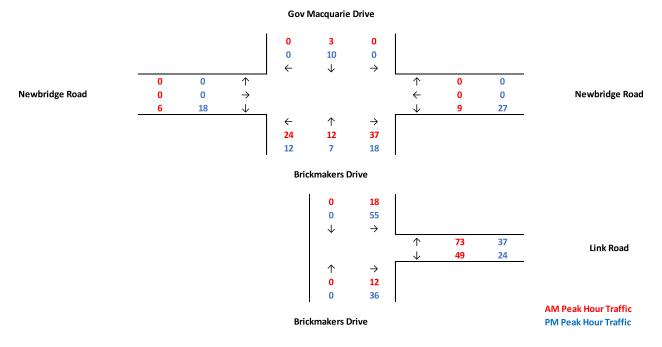


Figure 1 Traffic distribution for Moorebank Cove

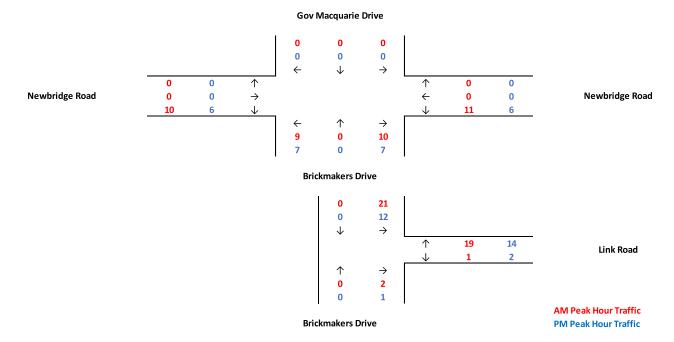


Figure 2 Traffic distribution for Moorebank Recycling Facility

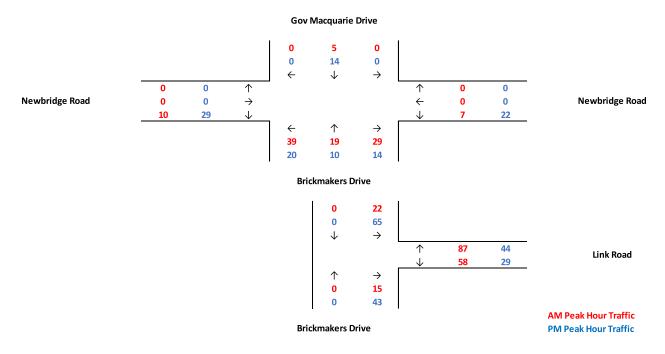


Figure 3 Traffic distribution for Georges Cove Marina (Residential)

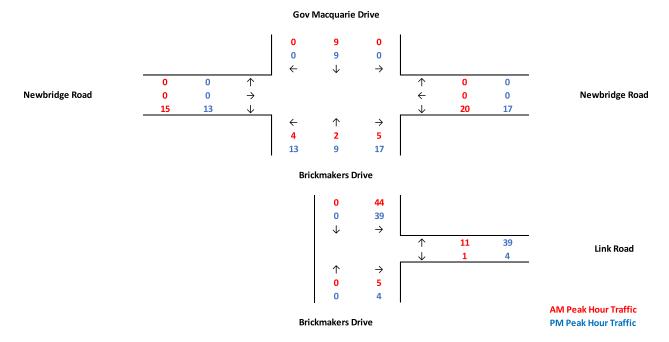


Figure 4 Traffic distribution for Georges Cove Marina (Commercial)

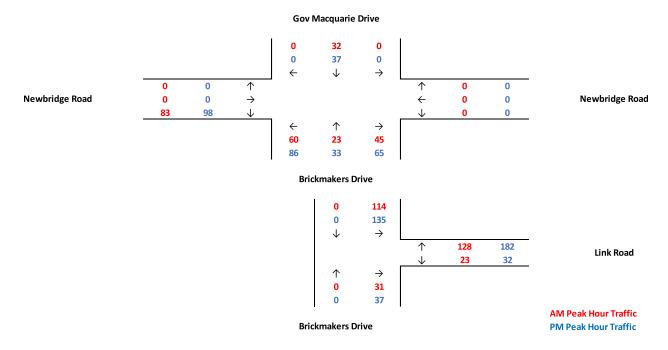


Figure 5 Traffic distribution for Benedict B6 mixed-use development

1.2.4 Overall traffic distribution

It has been assumed that all generated traffic from the Moorebank Recycling Facility are heavy vehicles while those from the rest of the developments are light vehicles. The total number of light and heavy vehicles from all developments are shown in Figure 6. The bracketed numbers represent heavy vehicle movements while the numbers unbracketed represent light vehicle movements.

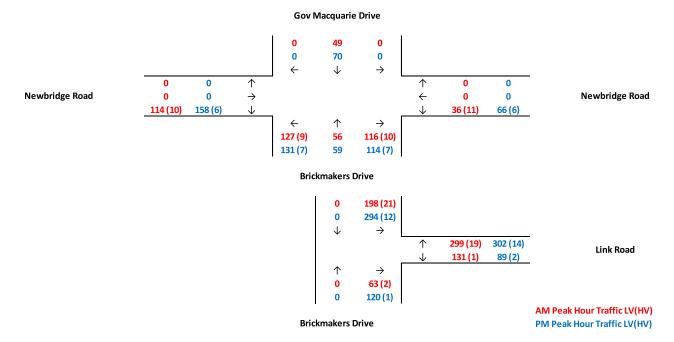


Figure 6 Traffic distribution

1.2.5 Existing and development traffic

As mentioned in Section 1.1, a new survey was conducted at the intersection of Newbridge Road with Governor Macquarie Drive and Brickmakers Drive as well as on Brickmakers Drive at the new Link Road. The existing surveyed traffic movements are presented in Figure 7.

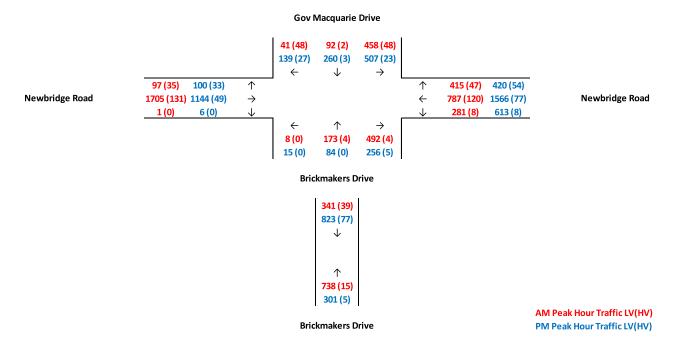


Figure 7 Existing traffic movements

The total movements from the exiting traffic movements combined with the additional traffic movements from all developments is presented in Figure 8.

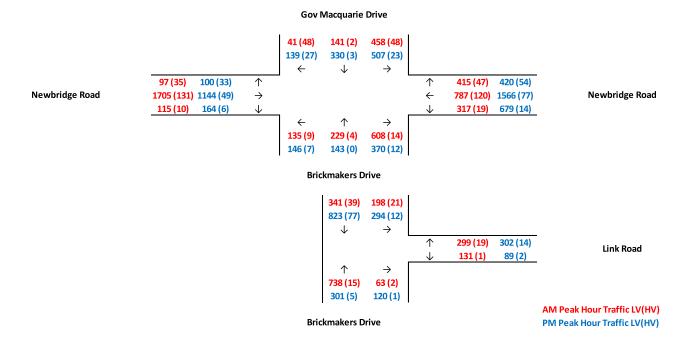


Figure 8 Total post-development traffic movements

1.3 Matter 3

RMS Comment:

The SIDRA model needs to be updated to include 140 second cycle times.

EMM Response:

The SIDRA model has been updated with 140 second cycle times. The roundabout option has also been explored in SIDRA as part of the sensitivity testing. Figure 9 and Figure 10 present the layout of the signalisation and roundabout respectively.

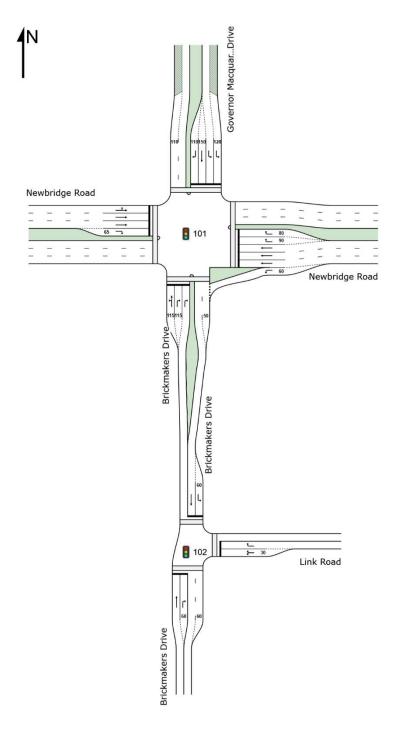


Figure 9 Signalised intersection layout

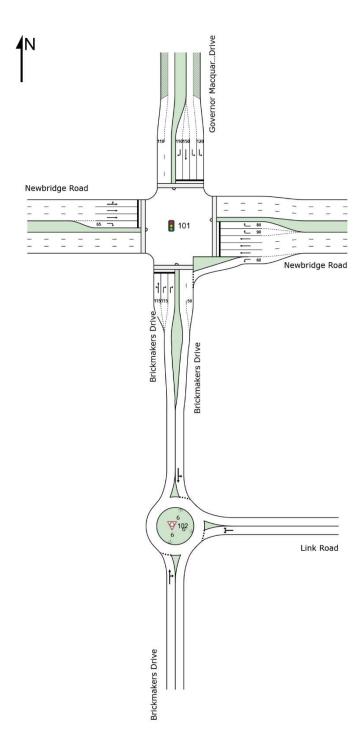


Figure 10 Roundabout intersection layout

Table 2 Summary of Brickmakers Drive/Link Road intersection performance during AM peak

Road	Signalised	intersection			Roundabo	out		
Approach	DOS	LOS	DEL	95 th %ile BQL	DOS	LOS	DEL	95 th %ile BQL
South: Brickn	nakers Drive	•						
Through	0.813	В	19.6	247.5	1.237	F	230.1	876.7
Right	0.165	В	20.4	16.4	1.237	F	232.8	876.7
East: Link Ro	ad							
Left	0.177	В	22.3	36.3	0.689	Α	11.6	45.1
Right	0.886	Е	66.2	172.9	0.689	А	14.0	45.1
North: Brickn	nakers Drive	:						
Left	0.146	Α	4.8	2.3	0.488	Α	4.9	32.3
Through	0.338	Α	7.5	48.4	0.488	А	4.4	32.3
All Vehicles	0.886	В	23.8	247.5	1.237	F	105.5	876.7

Note: DOS = degree of saturation, LOS = level of service, DEL = average delay by second, BQL = back of queue length Generally for priority controlled intersections, the longest delay occurs for the right turn movement from the minor road

Table 3 Summary of Brickmakers Drive/Link Road intersection performance during PM peak

	Signalised i	ntersection			Roundabo	ut		
	DOS	LOS	DEL	95 th %ile BQL	DOS	LOS	DEL	95 th %ile BQL
South: Brickn	nakers Drive							
Through	0.239	Α	9.8	57.6	0.524	Α	7.1	30.9
Right	0.535	В	25.8	40.0	0.524	Α	9.7	30.9
East: Link Roa	ad							
Left	0.177	С	34.2	34.7	0.829	С	34.7	107.3
Right	0.884	Е	68.7	168.6	0.829	С	37.0	107.3
North: Brickn	nakers Drive							
Left	0.180	Α	4.9	7.6	0.902	Α	8.4	148.9
Through	0.610	А	5.5	93.7	0.902	Α	8.1	148.9
All Vehicles	0.884	В	20.0	168.6	0.902	Α	14.3	148.9

The SIDRA results show that the roundabout option would have a LOS F with a 95th percentile queue length of 876.7 m in the south approach during the AM peak, which is beyond the roundabout at Conlon Avenue and results in a significant delay for traffic accessing the Brighton Lakes residential district. This is expected as the traffic flow on the three approaches will be disproportionate and the traffic on south approach will require to constantly give way to traffic turning right from the Link Road.

Signalisation of this intersection would result in a LOS E for right turning traffic from Link Road during the AM and PM peak hours with approximately 170 m queue length. However, the queue will be able to clear in during a single green-light (140 seconds) and thus is acceptable.

If the intersection is made a priority intersection with traffic on Link Road giving way to all traffic on Brickmakers Drive:

the high northbound traffic volume in the AM, and vice versa, would make it difficult/unsafe for traffic to turn right from Link Road.

The high right turn demand would also result in an exceedingly long queue on Link Road and subsequently jam up the internal road network. Hence, a priority control operation of Brickmakers Drive/Link Road is not a viable option.

1.4 Matter 4

RMS Comment:

It is noted that Roads and Maritime has previously requested a Warrants assessment for the new traffic control signals on Brickmakers Drive. This information is still outstanding and needs to be prepared in accordance with the traffic signal warrant (TCS) template. The applicant is requested to confirm that the minor road has +200 vehicles over a four hour period because the submitted information uses traffic data over a peak hour rather than over four hours.

EMM Response:

In accordance with the *Traffic Signal Design Guidelines*, the signal warrant requires the major road with a flow exceeding 600 vehicles/hour in both directions, or with a high speed limit/significant crash history. In terms of the major road traffic flow, the Brickmakers Drive/Link Road intersection will not generally meet the signal warrant.

However, given that the marina development is a major attraction for nearby residents, the marina will attract new pedestrian trips from the Brighton Lakes residential precinct on the other side of Brickmakers Drive, it is not practical to expect the increased number of pedestrians to take the 600 m detour to Newbridge Road and back just to cross the road. Further, there is no existing footpath on the eastern side of Brickmakers Drive. Therefore, upon development of the precinct, the lack of a pedestrian crossing along Brickmakers Drive would have significant pedestrian safety issues.

While a refuge island in Brickmakers Drive on pedestrians' desire line of travel it would require significant earthwork and road realignment due to constraints in the existing road width, the current cadastral boundaries and because this section of Brickmakers Drive is on a road bridge. Signalisation of the intersection requires minimal work and will provide the maximum safety for the increased number of pedestrians crossing Brickmakers Drive.

If the signals at Brickmakers Drive with its intersections of Newbridge Road and Link Road are coordinated, the maximum traffic flow could be achieved for both arterial and local roads.

Conclusions

This response letter summarises the post-development traffic volumes at the intersection of Brickmakers Drive with Link Road based on a traffic survey conducted in 2020. Sensitivity testing has been undertaken for the Brickmakers Drive/Link Road intersection based on the traffic operation and pedestrian safety. This found:

- priority intersection:
 - the high traffic volumes would result in a long queue on Link Road and the queue would back up the internal road network;
 - pedestrian access from the other side of Brickmakers Drive to the marina would require a long detour;
 - construction of refuge island is likely to require significant earthwork and road realignment due to existing constraints;
- roundabout intersection:
 - the disproportionate heavy traffic along Brickmakers Drive (heavy northbound AM peak and heavy southbound PM peak) would result in a long queue on the south approach of Brickmakers Drive in the AM peak and block access to the Brighton Lakes residential precinct at Conlon Avenue;
 - construction of roundabout with a refuge island is likely to require significant earthwork and road realignment due to existing constraints;
- Signalised intersection:
 - good intersection performance with acceptable queue lengths on all three approaches;
 - safest pedestrian access across Brickmakers Drive to the marina;
 - requires minimal works; and
 - future potential benefits if both of the adjoining signals are coordinated.

We trust this thoroughly addresses the RMS comments. Should you require further clarification regarding this matter, please do not hesitate to contact the undersigned.

Yours sincerely

Tim BrookerAssociate Director

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References

Ason Group. (2017). *Traffic Impact Assessment, Planning Proposal, Benedict Sands; 146 Newbridge Road, Moorebank.* Benedict Industries.

EMM Consulting. (2016). *Moorebank Cove Residential Estate Traffic Assessment*. Mirvac.

EMM Consulting. (2018). Georges Cove Marina Residential Planning Proposal. Mirvac.

Lyle Marshall & Associates. (2012). *Traffic report for construction and operation of a materials recycling facility on Lot 6, D.P. 1065574, Newbridge Road, Moorebank.* Concrete Recyclers Group.

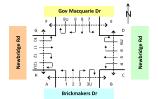
Appendix A

Intersection and tube count data

: N5560
:EMM
: Moorebank
: 1. Newbridge Rd / Gov Macquarie Dr / Brickmaker Job No. Client Suburb Location

: Thu, 30th January 2020 : Fine : Classified Intersection Count Day/Date Weather Description : 15 mins Data

Class 1 Class 2
Classifications Lights Heavies





Approach						Brickm	akers D											Newbr	dge Rd					
Direction		Direction (Left Turr			irection Through			Pirection Right Tur			irection 3 (U Turn)	BU		irection Left Turn			irection Through			irection Right Turi			irection 6 (U Turn)	
Time Period	ights	leavies	otal	sthgi	leavies	Fotal	lghts	Heavies	Total	ights	leavies	Total	ights	leavies	Total	lghts	Heavies	Total	ights	leavies	Fotal	ights	leavies	Total
7:00 to 7:15	1	0	1	33	2	35	134	1	135	0	0	0	70	1	71	200	30	230	101	14	115	0	0	0
7:15 to 7:30	2	0	2	41	1	42	124	0	124	0	0	0	60	1	61	181	39	220	113	16	129	0	0	0
7:30 to 7:45	4	0	4	42	0	42	88	0	88	0	0	0	63	0	63	194	26	220	60	6	66	0	0	0
7:45 to 8:00	1	0	1	57	1	58	146	3	149	0	0	0	88	6	94	212	25	237	141	11	152	0	0	0
8:00 to 8:15	3	0	3	72	0	72	161	2	163	0	0	0	65	0	65	164	25	189	125	14	139	0	0	0
8:15 to 8:30	6	0	6	72	1	73	117	1	118	0	0	0	64	1	65	149	23	172	152	12	164	0	0	0
8:30 to 8:45	2	0	2	64	1	65	108	2	110	0	0	0	58	2	60	159	29	188	118	13	131	0	0	0
8:45 to 9:00	3	0	3	49	1	50	109	1	110	0	0	0	64	1	65	196	32	228	97	15	112	0	0	0
AM Totals	22	0	22	430	7	437	987	10	997	0	0	0	532	12	544	1,455	229	1,684	907	101	1,008	0	0	0
16:00 to 16:15	4	0	4	25	1	26	102	4	106	0	0	0	165	2	167	411	19	430	121	12	133	0	0	0
16:15 to 16:30	2	1	3	21	0	21	112	0	112	0	0	0	151	1	152	332	19	351	91	15	106	0	0	0
16:30 to 16:45	2	0	2	24	0	24	73	0	73	0	0	0	146	2	148	306	23	329	92	17	109	0	0	0
16:45 to 17:00	5	0	5	26	0	26	54	1	55	0	0	0	162	1	163	381	24	405	125	11	136	0	0	0
17:00 to 17:15	4	0	4	16	0	16	77	2	79	0	0	0	166	2	168	411	14	425	89	13	102	0	0	0
17:15 to 17:30	4	0	4	18	0	18	52	2	54	0	0	0	139	3	142	468	16	484	114	13	127	1	0	1
17:30 to 17:45	1	0	1	18	0	18	55	0	55	0	0	0	145	0	145	398	17	415	109	12	121	0	0	0
17:45 to 18:00	1	0	1	24	0	24	46	0	46	0	0	0	159	0	159	383	17	400	141	15	156	0	0	0
PM Totals	23	1	24	172	1	173	571	9	580	0	0	0	1,233	11	1,244	3,090	149	3,239	882	108	990	1	0	1

Approach					G	ov Mac	quarie	Dr										Newb	ridge Ro	ı									Crossine				
Direction		Direction Left Turr			irection (Through			Direction Right Tur			irection 9 (U Turn)			irection : Left Turr			irection : (Through			irection Right Tur			irection 1 (U Turn)						edestria				
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	B to A	A to B	D to C	C to D	F to E	E to F	H to G	G to H	Total
7:00 to 7:15	113	14	127	24	0	24	10	14	24	0	0	0	16	10	26	389	23	412	0	0	0	0	0	0	0	0	0	0	0	0	3	1	4
7:15 to 7:30	98	11	109	21	1	22	11	8	19	0	0	0	26	8	34	506	43	549	1	0	1	0	0	0	0	0	0	0	0	0	1	0	1
7:30 to 7:45	130	14	144	19	0	19	9	14	23	0	0	0	24	6	30	435	33	468	0	0	0	0	0	0	0	0	0	0	0	0	1	1	2
7:45 to 8:00	117	9	126	28	1	29	11	12	23	0	0	0	31	11	42	375	32	407	0	0	0	0	0	0	0	0	0	0	0	0	3	1	4
8:00 to 8:15	133	16	149	26	5	31	8	10	18	0	0	0	26	17	43	294	32	326	0	1	1	1	0	1	1	0	0	0	1	0	6	1	9
8:15 to 8:30	142	16	158	19	0	19	14	13	27	0	0	0	38	11	49	316	36	352	1	0	1	0	0	0	0	1	0	1	0	0	8	2	12
8:30 to 8:45	135	22	157	45	1	46	19	15	34	0	0	0	51	10	61	295	44	339	0	1	1	0	0	0	0	0	0	0	0	0	4	0	4
8:45 to 9:00	72	26	98	26	2	28	25	11	36	0	0	0	49	10	59	316	49	365	5	0	5	0	0	0	0	0	0	1	0	0	2	1	4
AM Totals	940	128	1,068	208	10	218	107	97	204	0	0	0	261	83	344	2,926	292	3,218	7	2	9	1	0	1	1	1	0	2	1	0	28	7	40
16:00 to 16:15	194	24	218	64	0	64	41	11	52	0	0	0	2	0	2	1	0	1	0	0	0	0	0	0	1	0	1	0	0	0	0	6	8
16:15 to 16:30	197	14	211	59	0	59	35	10	45	0	0	0	6	3	9	178	14	192	1	0	1	1	0	1	1	0	4	0	3	2	0	0	10
16:30 to 16:45	140	3	143	63	0	63	31	6	37	0	0	0	23	12	35	351	15	366	0	0	0	0	0	0	0	0	0	0	0	1	1	1	3
16:45 to 17:00	105	8	113	51	2	53	20	9	29	0	0	0	25	6	31	209	11	220	1	0	1	0	0	0	1	0	1	1	0	3	1	1	8
17:00 to 17:15	153	9	162	79	1	80	42	5	47	0	0	0	27	7	34	276	11	287	3	0	3	0	0	0	0	0	0	0	0	1	0	0	1
17:15 to 17:30	109	3	112	67	0	67	46	7	53	0	0	0	25	8	33	308	12	320	2	0	2	0	0	0	0	0	0	0	0	0	0	1	1
17:30 to 17:45	109	4	113	53	0	53	29	5	34	0	0	0	14	8	22	311	15	326	2	0	2	0	0	0	0	0	0	0	0	0	0	0	0
17:45 to 18:00	80	3	83	37	0	37	26	8	34	0	0	0	24	- 6	30	234	12	246	2	0	2	0	0	0	0	0	1	0	0	0	0	0	1
PM Totals	1,087	68	1,155	473	3	476	270	61	331	٥	0	0	146	50	196	1,868	90	1,958	11	0	11	1	0	1	3	0	7	1	3	7	2	9	32

Job No. Client Suburb Location : N5560 : EMM

: Moorebank : 1. Newbridge Rd / Gov Macquarie Dr / Brickmakers Dr

Day/Date : Thu, 30th January 2020

Weather Description

: Fine : Classified Intersection Count : Hourly Summary

Newbridge Rd	Gov Macquarie Dr
	Brickmakers Dr



Approach						Brickm	akers D	,										Newbr	idge Rd					
Direction		Direction Left Turn			irection [Through]			Direction Right Tur			irection : (U Turn)			irection Left Turr			irection Through			irection Right Tur			irection ((U Turn)	
Time Period	Lights	Heavies	Fotal	Lights	Heavies	Fotal	Lights	Heavies	Total	Lights	Heavies	Fotal	Lights	Heavies	Fotal	Lights	Heavies	Fotal	Lights	Heavies	Fotal	Lights	Heavies	Fotal
7:00 to 8:00	8	0	8	173	4	177	492	4	496	0	0	0	281	8	289	787	120	907	415	47	462	0	0	0
7:15 to 8:15	10	0	10	212	2	214	519	5	524	0	0	0	276	7	283	751	115	866	439	47	486	0	0	0
7:30 to 8:30	14	0	14	243	2	245	512	6	518	0	0	0	280	7	287	719	99	818	478	43	521	0	0	0
7:45 to 8:45	12	0	12	265	3	268	532	8	540	0	0	0	275	9	284	684	102	786	536	50	586	0	0	0
8:00 to 9:00	14	0	14	257	3	260	495	6	501	0	0	0	251	4	255	668	109	777	492	54	546	0	0	0
AM Totals	22	0	22	430	7	437	987	10	997	0	0	0	532	12	544	1,455	229	1,684	907	101	1,008	0	0	0
16:00 to 17:00	13	1	14	96	1	97	341	5	346	0	0	0	624	6	630	1,430	85	1,515	429	55	484	0	0	0
16:15 to 17:15	13	1	14	87	0	87	316	3	319	0	0	0	625	6	631	1,430	80	1,510	397	56	453	0	0	0
16:30 to 17:30	15	0	15	84	0	84	256	5	261	0	0	0	613	8	621	1,566	77	1,643	420	54	474	1	0	1
16:45 to 17:45	14	0	14	78	0	78	238	5	243	0	0	0	612	6	618	1,658	71	1,729	437	49	486	1	0	1
17:00 to 18:00	10	0	10	76	0	76	230	4	234	0	0	0	609	5	614	1,660	64	1,724	453	53	506	1	0	1
PM Totals	23	1	24	172	1	173	571	9	580	0	0	0	1,233	11	1,244	3,090	149	3,239	882	108	990	1	0	1

Direction		Direction Left Turn			irection Through			Direction Right Tur			irection 9 (U Turn)			rection : Left Turr			irection (Through			irection : Right Tur			rection 1 (U Turn)					P	edestria	ns			
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	B to A	A to B	D to C	C to D	F to E	E to F	H to G	G to H	Total
7:00 to 8:00	458	48	506	92	2	94	41	48	89	0	0	0	97	35	132	1,705	131	1,836	1	0	1	0	0	0	0	0	0	0	0	0	8	3	11
7:15 to 8:15	478	50	528	94	7	101	39	44	83	0	0	0	107	42	149	1,610	140	1,750	1	1	2	1	0	1	1	0	0	0	1	0	11	3	16
7:30 to 8:30	522	55	577	92	6	98	42	49	91	0	0	0	119	45	164	1,420	133	1,553	1	1	2	1	0	1	1	1	0	1	1	0	18	5	27
7:45 to 8:45	527	63	590	118	7	125	52	50	102	0	0	0	146	49	195	1,280	144	1,424	1	2	3	1	0	1	1	1	0	1	1	0	21	4	29
8:00 to 9:00	482	80	562	116	8	124	66	49	115	0	0	0	164	48	212	1,221	161	1,382	6	2	8	1	0	1	1	1	0	2	1	0	20	4	29
AM Totals	940	128	1,068	208	10	218	107	97	204	0	0	0	261	83	344	2,926	292	3,218	7	2	9	1	0	1	1	1	0	2	1	0	28	7	40
16:00 to 17:00	636	49	685	237	2	239	127	36	163	0	0	0	56	21	77	739	40	779	2	0	2	1	0	1	3	0	6	1	3	6	2	8	29
16:15 to 17:15	595	34	629	252	3	255	128	30	158	0	0	0	81	28	109	1,014	51	1,065	5	0	5	1	0	1	2	0	5	1	3	7	2	2	22
16:30 to 17:30	507	23	530	260	3	263	139	27	166	0	0	0	100	33	133	1,144	49	1,193	6	0	6	0	0	0	1	0	1	1	0	5	2	3	13
16:45 to 17:45	476	24	500	250	3	253	137	26	163	0	0	0	91	29	120	1,104	49	1,153	8	0	8	0	0	0	1	0	1	1	0	4	1	2	10
17:00 to 18:00	451	19	470	236	1	237	143	25	168	0	0	0	90	29	119	1,129	50	1,179	9	0	9	0	0	0	0	0	1	0	0	1	0	1	3
PM Totals	1,087	68	1,155	473	3	476	270	61	331	0	0	0	146	50	196	1,868	90	1,958	11	0	11	1	0	1	3	0	7	1	3	7	2	9	32

Job No N5560 Client Eric Lei

Site Brickmakers Drive
Location Mooorebank

Site No 1

Start Date 30-Jan-20

Description Volume Summary

Direction Combined



			D	ay of We	ek				
Hour	Mon	Tue	Wed	Thu	Fri	Sat	Sun		
Starting	3-Feb	4-Feb	5-Feb	30-Jan	31-Jan	1-Feb	2-Feb	W'Day	7 Day
AM Peak	1152	1099	1092	1122	1200	882	710	Ave	Ave
PM Peak	1217	1228	1259	1313	1203	919	772	14929	13700
0:00	81	83	79	82	98	167	196	85	112
1:00	42	51	52	34	31	95	103	42	58
2:00	30	25	29	38	37	70	77	32	44
3:00	35	43	46	56	53	72	59	47	52
4:00	141	139	136	148	141	86	60	141	122
5:00	460	454	443	448	432	188	100	447	361
6:00	800	853	850	828	806	357	161	827	665
7:00	1030	879	1086	996	994	433	255	997	810
8:00	1152	1099	1092	1122	1200	609	343	1133	945
9:00	778	811	942	785	795	772	513	822	771
10:00	579	598	624	631	669	882	653	620	662
11:00	551	634	593	674	670	838	710	624	667
12:00	600	700	671	616	722	919	772	662	714
13:00	657	712	727	735	782	795	736	723	735
14:00	970	983	963	1076	1012	724	651	1001	911
15:00	1217	1187	1189	1306	1203	646	674	1220	1060
16:00	1162	1228	1135	1313	1179	615	691	1203	1046
17:00	1211	1197	1259	1180	1182	651	642	1206	1046
18:00	969	1024	1039	975	931	655	593	988	884
19:00	666	666	662	661	701	541	420	671	617
20:00	497	520	553	568	536	451	400	535	504
21:00	343	445	478	446	486	444	325	440	424
22:00	231	289	278	292	363	399	253	291	301
23:00	152	128	168	136	280	317	143	173	189
Total	14354	14748	15094	15146	15303	11726	9530	14929	13700
7-19	10876	11052	11320	11409	11339	8539	7233	11199	10253

7-19	10876	11052	11320	11409	11339	8539	7233	11199	10253
6-22	13182	13536	13863	13912	13868	10332	8539	13672	12462
6-24	13565	13953	14309	14340	14511	11048	8935	14136	12952
0-24	14354	14748	15094	15146	15303	11726	9530	14929	13700

Appendix B

SIDRA results (signalisation)

Site: 101 [Dev Newbridge Rd/Gov Macquarie Dr/Brickmakers Dr AM]

Dev Four Way Intersection Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 140 seconds (Network User-Given Cycle Time)

Mov	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued	Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m		rate		km/h
Sout	h: Brick	makers Dri	ive											
1	L2	151	6.3	151	6.3	1.075	159.8	LOS F	44.5	320.5	1.00	1.43	1.85	13.8
2	T1	236	1.7	236	1.7	1.075	155.2	LOS F	44.5	320.5	1.00	1.43	1.85	14.0
3	R2	629	2.3	629	2.3	1.047	135.5	LOS F	32.5	231.9	1.00	1.24	1.69	15.7
Appr	oach	1016	2.8	1016	2.8	1.075	143.7	LOS F	44.5	320.5	1.00	1.31	1.75	15.0
East	: Newbr	idge Road												
4	L2	338	5.8	338	5.8	0.337	11.7	LOS A	6.0	43.9	0.43	0.71	0.43	48.9
5	T1	907	13.2	907	13.2	0.327	21.0	LOS B	11.8	92.2	0.63	0.54	0.63	50.1
6	R2	462	10.2	462	10.2	1.897	611.5	LOS F	66.4	505.4	1.00	1.65	3.53	5.1
Appr	roach	1707	10.9	1707	10.9	1.897	178.9	LOS F	66.4	505.4	0.69	0.88	1.37	13.8
Nort	h: Gove	rnor Macqı	uarie D	rive										
7	L2	506	9.5	506	9.5	0.452	36.9	LOS C	9.0	68.4	0.84	0.86	1.02	36.9
8	T1	146	1.4	146	1.4	0.391	54.7	LOS D	8.7	61.8	0.92	0.76	0.92	22.1
9	R2	89	53.9	89	53.9	0.331	40.0	LOS C	4.0	41.0	0.91	0.76	0.91	32.1
Appr	oach	741	13.2	741	13.2	0.452	40.7	LOS C	9.0	68.4	0.86	0.83	0.99	33.7
Wes	t: Newb	ridge Road												
10	L2	132	26.5	132	26.5	1.090	158.1	LOS F	76.7	587.3	1.00	1.43	1.83	16.0
11	T1	1836	7.1	1836	7.1	1.090	158.3	LOS F	84.8	630.2	1.00	1.51	1.84	16.8
12	R2	132	8.0	132	8.0	0.282	43.3	LOS D	6.6	49.2	0.78	0.78	0.78	26.8
Appr	oach	2100	8.4	2100	8.4	1.090	151.1	LOS F	84.8	630.2	0.99	1.46	1.77	17.0
All V	ehicles	5563	8.8	5563	8.8	1.897	143.6	LOS F	84.8	630.2	0.88	1.17	1.54	16.5

♦♦ Network: N101 [Dev AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Ped	estrians						
Mov I D	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	21	24.6	LOS C	0.0	0.0	0.59	0.59
P2	East Full Crossing	11	64.2	LOS F	0.0	0.0	0.96	0.96
P3	North Full Crossing	21	42.5	LOS E	0.1	0.1	0.78	0.78
P4	West Full Crossing	53	63.3	LOS F	0.2	0.2	0.95	0.95
All Pe	destrians	105	51.5	LOS E			0.85	0.85

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 102 [Dev Brickmakers Dr/Link Rd AM]

New Intersection with Traffic Signals

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 140 seconds (Network User-Given Cycle Time)

Move	ement	Performa	ance -	Vehic	es									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	ı: Brickı	makers Dri	ve											
2	T1	753	2.0	753	2.0	0.813	19.6	LOS B	34.8	247.5	0.75	0.69	0.75	32.6
3	R2	69	3.0	69	3.0	0.165	20.4	LOS B	2.3	16.4	0.51	0.68	0.51	38.8
Appro	ach	822	2.1	822	2.1	0.813	19.7	LOS B	34.8	247.5	0.73	0.69	0.73	33.5
East:	Link Ro	oad												
4	L2	140	0.8	140	8.0	0.177	22.3	LOS B	5.1	36.3	0.58	0.70	0.58	38.0
6	R2	336	6.0	336	6.0	0.886	66.2	LOS E	23.5	172.9	0.93	0.95	1.17	17.9
Appro	ach	476	4.4	476	4.4	0.886	53.3	LOS D	23.5	172.9	0.83	0.87	1.00	23.6
North	: Brickr	nakers Dri	ve											
7	L2	232	9.5	232	9.5	0.146	4.8	LOS A	0.3	2.3	0.04	0.54	0.04	45.6
8	T1	380	10.3	380	10.3	0.338	7.5	LOS A	6.4	48.4	0.27	0.24	0.27	44.3
Appro	ach	612	10.0	612	10.0	0.338	6.5	LOS A	6.4	48.4	0.18	0.35	0.18	44.8
All Ve	hicles	1910	5.2	1910	5.2	0.886	23.8	LOS B	34.8	247.5	0.58	0.63	0.62	33.3

♦ Network: N101 [Dev AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pe	destrians						
Mov I D	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	11	41.7	LOS E	0.0	0.0	0.77	0.77
P2	East Full Crossing	21	13.3	LOS B	0.0	0.0	0.44	0.44
P3	North Full Crossing	53	39.5	LOS D	0.2	0.2	0.75	0.75
All Pe	destrians	84	33.2	LOS D			0.68	0.68

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [Dev Newbridge Rd/Gov Macquarie Dr/Brickmakers Dr PM]

Dev Four Way Intersection Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 140 seconds (Network User-Given Cycle Time)

Mov	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued	Effective A Stop Rate	ver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
Sout	h: Brick	makers Dri	ve											
1	L2	160	4.6	160	4.6	1.044	134.3	LOS F	30.1	215.4	1.00	1.22	1.58	15.8
2	T1	146	0.0	146	0.0	1.044	129.7	LOS F	30.1	215.4	1.00	1.22	1.58	15.9
3	R2	388	3.2	388	3.2	0.554	60.1	LOS E	12.6	90.3	0.99	0.83	0.99	26.7
Appr	oach	695	2.8	695	2.8	1.044	91.9	LOS F	30.1	215.4	1.00	1.00	1.25	20.5
East	Newbr	idge Road												
4	L2	697	2.1	697	2.1	1.304	334.7	LOS F	115.8	824.6	1.00	1.56	2.70	4.9
5	T1	1643	4.7	1643	4.7	0.692	24.5	LOS B	34.3	250.0	0.75	0.67	0.75	47.8
6	R2	474	11.4	474	11.4	1.121	121.1	LOS F	28.5	218.9	1.00	1.09	1.69	18.3
Appr	oach	2814	5.2	2814	5.2	1.304	117.6	LOS F	115.8	824.6	0.85	0.96	1.39	18.6
North	n: Gove	rnor Macqu	ıarie D	rive										
7	L2	530	4.3	530	4.3	0.420	31.9	LOS C	8.5	61.6	0.81	0.84	0.93	39.5
8	T1	337	0.9	337	0.9	1.105	179.0	LOS F	41.1	289.6	1.00	1.50	1.99	8.8
9	R2	166	16.3	166	16.3	0.443	39.9	LOS C	7.7	61.8	0.91	0.79	0.91	35.5
Appr	oach	1033	5.1	1033	5.1	1.105	81.1	LOS F	41.1	289.6	0.89	1.04	1.27	22.9
West	: Newb	ridge Road												
10	L2	133	24.8	133	24.8	0.890	68.7	LOS E	34.0	258.8	1.00	1.02	1.29	29.8
11	T1	1193	4.1	1193	4.1	0.890	62.5	LOS E	35.8	259.3	0.99	1.00	1.21	31.9
12	R2	179	3.5	179	3.5	0.710	68.4	LOS E	12.0	86.7	1.00	0.85	1.06	19.7
Appr	oach	1505	5.9	1505	5.9	0.890	63.7	LOS E	35.8	259.3	0.99	0.98	1.20	30.5
All Ve	ehicles	6046	5.1	6046	5.1	1.304	95.0	LOS F	115.8	824.6	0.91	0.98	1.31	21.7

♦♦ Network: N102 [Dev PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Ped	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	21	24.0	LOS C	0.0	0.0	0.59	0.59
P2	East Full Crossing	11	64.2	LOS F	0.0	0.0	0.96	0.96
P3	North Full Crossing	21	48.9	LOS E	0.1	0.1	0.84	0.84
P4	West Full Crossing	53	64.3	LOS F	0.2	0.2	0.96	0.96
All Pe	destrians	105	53.1	LOS E			0.86	0.86

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 102 [Dev Brickmakers Dr/Link Rd PM]

New Intersection with Traffic Signals

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 140 seconds (Network User-Given Cycle Time)

Move	ement	Performa	ance -	Vehic	es									
Mov ID	Turn	Demand Total	Flows HV		Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	ı: Bricki	makers Dri	ive											
2	T1	306	1.6	306	1.6	0.239	9.8	LOS A	8.1	57.6	0.43	0.37	0.43	39.5
3	R2	128	0.8	128	8.0	0.535	25.8	LOS B	5.7	40.0	0.67	0.76	0.67	36.7
Appro	oach	434	1.4	434	1.4	0.535	14.5	LOS B	8.1	57.6	0.50	0.49	0.50	38.2
East:	Link R	oad												
4	L2	97	2.2	97	2.2	0.177	34.2	LOS C	4.9	34.7	0.73	0.73	0.73	33.8
6	R2	334	4.4	334	4.4	0.884	68.7	LOS E	23.2	168.6	0.96	0.94	1.19	17.5
Appro	oach	430	3.9	430	3.9	0.884	60.9	LOS E	23.2	168.6	0.91	0.90	1.09	21.2
North	: Brickr	nakers Dri	ve											
7	L2	323	3.9	298	3.7	0.180	4.9	LOS A	1.0	7.6	0.11	0.56	0.11	45.5
8	T1	900	8.6	741	10.1	0.610	5.5	LOS A	12.3	93.7	0.27	0.25	0.27	45.7
Appro	oach	1223	7.3	1039 ^N	8.3	0.610	5.3	LOS A	12.3	93.7	0.22	0.34	0.22	45.7
All Ve	hicles	2088	5.4	1904 ^N	¹¹ 5.9	0.884	20.0	LOS B	23.2	168.6	0.44	0.50	0.48	36.2

+ Network: N102 [Dev PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Move	ement Performance - Pe	edestrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	11	47.3	LOS E	0.0	0.0	0.82	0.82
P2	East Full Crossing	21	10.4	LOS B	0.0	0.0	0.39	0.39
P3	North Full Crossing	53	44.9	LOS E	0.2	0.2	0.80	0.80
All Pe	destrians	84	36.6	LOS D			0.70	0.70

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Appendix C

SIDRA results (roundabout)

Site: 101 [Dev Newbridge Rd/Gov Macquarie Dr/Brickmakers Dr AM]

Dev Four Way Intersection Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 140 seconds (Network User-Given Cycle Time)

Мо	vement	Performa	ince -	Vehic	es									
Mov ID	/ Turn	Demand Total		Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued	Effective / Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
Sou	ıth: Brick	makers Dri	ve											
1	L2	151	6.3	150	6.3	1.202	259.7	LOS F	52.2	376.6	1.00	1.76	2.37	9.2
2	T1	236	1.7	202	1.6	1.202	255.0	LOS F	52.2	376.6	1.00	1.76	2.37	9.3
3	R2	629	2.3	533	2.6	0.819	69.9	LOS E	18.8	134.9	1.00	0.92	1.14	24.6
App	roach	1016	2.8	885 ^N	3.0	1.202	144.2	LOS F	52.2	376.6	1.00	1.26	1.63	14.9
Eas	t: Newbr	idge Road												
4	L2	338	5.8	338	5.8	0.336	11.7	LOS A	6.0	43.8	0.43	0.71	0.43	48.9
5	T1	907	13.2	907	13.2	0.314	19.1	LOS B	11.3	88.1	0.60	0.52	0.60	51.3
6	R2	462	10.2	462	10.2	1.897	611.5	LOS F	66.0	502.0	1.00	1.65	3.53	5.1
App	roach	1707	10.9	1707	10.9	1.897	177.9	LOS F	66.0	502.0	0.67	0.86	1.36	13.8
Nor	th: Gove	rnor Macqu	ıarie D	rive										
7	L2	506	9.5	506	9.5	0.509	40.9	LOS C	9.4	71.5	0.89	0.88	1.08	35.5
8	T1	146	1.4	146	1.4	0.479	60.0	LOS E	9.2	65.0	0.96	0.79	0.96	20.9
9	R2	89	53.9	89	53.9	0.309	40.8	LOS C	4.1	42.3	0.89	0.76	0.89	31.8
App	roach	741	13.2	741	13.2	0.509	44.6	LOS D	9.4	71.5	0.90	0.85	1.04	32.4
We	st: Newb	ridge Road												
10	L2	132	26.5	132	26.5	1.028	112.5	LOS F	65.2	499.0	1.00	1.26	1.56	20.3
11	T1	1836	7.1	1836	7.1	1.028	113.2	LOS F	73.0	542.2	1.00	1.32	1.57	21.5
12	R2	132	8.0	132	8.0	0.263	40.8	LOS C	6.3	47.5	0.75	0.77	0.75	27.8
App	roach	2100	8.4	2100	8.4	1.028	108.6	LOS F	73.0	542.2	0.98	1.28	1.52	21.6
All ۷	√ehic l es	5563	8.8	<mark>5433</mark> N	9.0	1.897	127.5	LOS F	73.0	542.2	0.88	1.09	1.42	18.1

♦♦ Network: N101 [Dev AM

roundabout]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Move	ement Performance - Pe	destrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	21	22.9	LOS C	0.0	0.0	0.57	0.57
P2	East Full Crossing	11	64.2	LOS F	0.0	0.0	0.96	0.96
P3	North Full Crossing	21	40.2	LOS E	0.1	0.1	0.76	0.76
P4	West Full Crossing	53	64.3	LOS F	0.2	0.2	0.96	0.96

All Pedestrians 105 51.2 LOS E 0.84 0.84

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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₩ Site: 102 [Dev Brickmakers Dr/Link Rd AM roundabout]

♦♦ Network: N101 [Dev AM roundabout]

New Roundabout Site Category: (None) Roundabout

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles		Prop. Queued	Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	ı: Brickr	makers Dri	ve											
2	T1	793	2.0	793	2.0	1.237	230.1	LOS F	123.1	876.7	1.00	5.37	9.52	6.9
3	R2	69	3.0	69	3.0	1.237	232.8	LOS F	123.1	876.7	1.00	5.37	9.52	12.0
Appro	oach	862	2.1	862	2.1	1.237	230.3	LOS F	123.1	876.7	1.00	5.37	9.52	7.4
East:	Link Ro	oad												
4	L2	140	0.8	140	0.8	0.689	11.6	LOS A	6.2	45.1	0.78	0.89	1.00	41.7
6	R2	336	6.0	336	6.0	0.689	14.0	LOS A	6.2	45.1	0.78	0.89	1.00	36.9
Appro	ach	476	4.4	476	4.4	0.689	13.3	LOS A	6.2	45.1	0.78	0.89	1.00	38.9
North	: Brickn	nakers Dri	ve											
7	L2	232	9.5	232	9.5	0.488	4.9	LOS A	4.2	32.3	0.30	0.48	0.30	44.7
8	T1	400	10.3	400	10.3	0.488	4.4	LOS A	4.2	32.3	0.30	0.48	0.30	45.4
Appro	ach	632	10.0	632	10.0	0.488	4.6	LOS A	4.2	32.3	0.30	0.48	0.30	45.2
All Ve	hicles	1969	5.2	1969	5.2	1.237	105.5	LOS F	123.1	876.7	0.72	2.72	4.51	15.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Dev Newbridge Rd/Gov Macquarie Dr/Brickmakers Dr PM]

Dev Four Way Intersection Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 140 seconds (Network User-Given Cycle Time)

Mov	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		l Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A	ver. No.A Cycles S	
		veh/h	0/2	veh/h	%	v/c	sec		veh	m		Rate		km/h
Sout	h: Brick	makers Dri		VC11/11	/0	V/C	360		Ven					KIII/II
1	L2	160	4.6	160	4.6	1.044	139.6	LOS F	32.3	230.7	1.00	1.32	1.76	15.4
2	T1	146	0.0	146	0.0	1.044	135.0	LOS F	32.3	230.7	1.00	1.32	1.76	15.5
3	R2	388	3.2	388	3.2	0.554	60.0	LOS E	12.1	87.0	0.96	0.81	0.96	26.7
Appr	oach	695	2.8	695	2.8	1.044	94.1	LOS F	32.3	230.7	0.98	1.04	1.31	20.2
East	: Newbr	idge Road												
4	L2	697	2.1	697	2.1	1.304	334.7	LOS F	115.8	824.6	1.00	1.56	2.70	4.9
5	T1	1643	4.7	1643	4.7	0.692	24.5	LOS B	34.3	250.0	0.75	0.67	0.75	47.8
6	R2	474	11.4	474	11.4	1.121	121.1	LOS F	28.5	218.9	1.00	1.09	1.69	18.3
Appr	oach	2814	5.2	2814	5.2	1.304	117.6	LOS F	115.8	824.6	0.85	0.96	1.39	18.6
Nort	h: Gove	rnor Macqu	ıarie D	rive										
7	L2	530	4.3	530	4.3	0.420	31.9	LOS C	8.5	61.6	0.81	0.84	0.93	39.5
8	T1	337	0.9	337	0.9	1.105	179.0	LOS F	41.1	289.6	1.00	1.50	1.99	8.8
9	R2	166	16.3	166	16.3	0.443	39.9	LOS C	7.7	61.8	0.91	0.79	0.91	35.5
Appr	oach	1033	5.1	1033	5.1	1.105	81.1	LOS F	41.1	289.6	0.89	1.04	1.27	22.9
Wes	t: Newb	ridge Road												
10	L2	133	24.8	133	24.8	0.890	68.7	LOS E	34.0	258.8	1.00	1.02	1.29	29.8
11	T1	1193	4.1	1193	4.1	0.890	62.5	LOS E	35.8	259.3	0.99	1.00	1.21	31.9
12	R2	179	3.5	179	3.5	0.710	68.4	LOS E	12.0	86.7	1.00	0.85	1.06	19.7
Appr	roach	1505	5.9	1505	5.9	0.890	63.7	LOS E	35.8	259.3	0.99	0.98	1.20	30.5
All V	ehic l es	6046	5.1	6046	5.1	1.304	95.2	LOS F	115.8	824.6	0.91	0.99	1.31	21.7

♦♦ Network: N102 [Dev PM

roundabout]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians											
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate			
P1	South Full Crossing	21	24.0	LOS C	0.0	0.0	0.59	0.59			
P2	East Full Crossing	11	64.2	LOS F	0.0	0.0	0.96	0.96			
P3	North Full Crossing	21	48.9	LOS E	0.1	0.1	0.84	0.84			
P4	West Full Crossing	53	64.3	LOS F	0.2	0.2	0.96	0.96			
All Pedestrians		105	53.1	LOS E			0.86	0.86			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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₩ Site: 102 [Dev Brickmakers Dr/Link Rd PM roundabout]

♦♦ Network: N102 [Dev PM roundabout]

New Roundabout Site Category: (None) Roundabout

Movement Performance - Vehicles														
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles		Prop. Queued	Effective A Stop Rate	ver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
South: Brickmakers Drive														
2	T1	322	1.6	322	1.6	0.524	7.1	LOS A	4.4	30.9	0.76	0.77	0.79	41.2
3	R2	128	0.8	128	0.8	0.524	9.7	LOS A	4.4	30.9	0.76	0.77	0.79	44.6
Appro	ach	451	1.4	451	1.4	0.524	7.8	LOS A	4.4	30.9	0.76	0.77	0.79	42.6
East: Link Road														
4	L2	97	2.2	97	2.2	0.829	34.7	LOS C	14.8	107.3	1.00	1.38	1.97	33.0
6	R2	334	4.4	334	4.4	0.829	37.0	LOS C	14.8	107.3	1.00	1.38	1.97	25.3
Appro	ach	431	3.9	431	3.9	0.829	36.4	LOS C	14.8	107.3	1.00	1.38	1.97	27.7
North: Brickmakers Drive														
7	L2	323	3.9	298	3.7	0.902	8.4	LOS A	19.8	148.9	0.74	0.62	0.79	43.0
8	T1	947	8.6	777	10.1	0.902	8.1	LOS A	19.8	148.9	0.74	0.62	0.79	43.5
Appro	ach	1271	7.4	1075 ^N	8.4	0.902	8.1	LOS A	19.8	148.9	0.74	0.62	0.79	43.4
All Ve	hicles	2152	5.4	1956 ^N	6.0	0.902	14.3	LOSA	19.8	148.9	0.80	0.82	1.05	39.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Appendix D

RMS (Now TfNSW) Traffic Signal Warrant Guidelines



2. I Introduction

This section describes the general warrants for the installation of traffic signals. It must be emphasised that these are only a guide. If a site satisfies the warrants, it does not necessarily mean that traffic signals are the best solution. All traffic data should be analysed and alternative treatments considered to determine the optimum solution (see Section 4 of the Road Design Guide). Traffic signals are sometimes installed due to public pressure or an administrative directive irrespective of the general warrants.

2.2 FACTORS INFLUENCING THE PROVISION OF TRAFFIC SIGNALS

Traffic signals are usually installed at an intersection:

- to provide traffic control at a site with a traffic capacity or road safety problem
- to control conflicting movements with high traffic flows
- to facilitate access to and from local areas in a major/minor road system, including pedestrian movements
- as part of an area wide system of traffic management

A side effect of signalisation is that the traffic flow on a major road is broken up into platoons. This assists nearby pedestrians to cross the major road and vehicles in nearby side streets to cross or enter the major road.

Factors influencing the provision of traffic signals include:

- traffic flows
- traffic conflicts
- traffic accident statistics
- pedestrian requirements
- access to major roads
- cost of installation
- availability of fundsmaintenance costs
- practicality
- feasibility
- the signposted speed limit is not more than 80km/h

General warrants are given in the following sub-sections. The figures stated should only be used as a guide and each intersection should be considered in more detail before being accepted for signal design.

2.3 SIGNALISED INTERSECTIONS

As a guide, a signalised intersection may be considered if one of the following warrants is met.

(a) Traffic demand:

For each of four one-hour periods of an average day:

- (i) the major road flow exceeds 600 vehicles/hour in each direction; and
- (ii) the minor road flow exceeds 200 vehicles/hour in one direction.

OR



(b) Continuous traffic:

For each of four one-hour periods of an average day:

- (i) the major road flow exceeds 900 vehicles/hour in each direction; and
- (ii) the minor road flow exceeds 100 vehicles/hour in one direction; and
- (iii) the speed of traffic on the major road or limited sight distance from the minor road causes undue delay or hazard to the minor road vehicles; and
- (iv) there is no other nearby traffic signal site easily accessible to the minor road vehicles.

OR

(c) Pedestrian safety:

For each of four one-hour periods of an average day:

- (i) the pedestrian flow crossing the major road exceeds 150 persons/hour; and
- (ii) the major road flow exceeds 600 vehicles/hour in each direction or, where there is a central median of at least 1.2 m wide, 1000 vehicles/hour in each direction.

OR

(d) Pedestrian safety - high speed road:

For each of four one-hour periods of an average day:

- (i) the pedestrian flow crossing the major road exceeds 150 persons/hour; and
- the major road flow exceeds 450 vehicles/hour in each direction or, where there is a central median of at least 1.2 m wide, 750 vehicles/hour in each direction; and
- (iii) the 85th percentile speed on the major road exceeds 75 km/h.

OR

(e) Crashes:

- (i) The intersection has been the site of an average of three or more reported tow-away or casualty traffic accidents per year over a three year period, where the traffic accidents could have been prevented by traffic signals; and
- (ii) the traffic flows are at least 80% of the appropriate flow warrants.

2.4 SIGNALISED MARKED FOOT CROSSINGS AT INTERSECTIONS

A signalised marked foot crossing must be provided on each leg of a signalised intersection (including T Junctions), in a built-up area, except in the following circumstances:

- (a) There are significant road safety implications:
 - (i) there is insufficient sight distance (see Section 4 of the Road Design Guide); or
 - (ii) there is adverse road geometry (see Section 4 of the Road Design Guide).
- (b) There are significant adverse transport efficiency implications
 - (i) there is an unacceptable increase in delay and degree of saturation which must be substantiated by intersection modelling; or